exists. Although inexperienced drivers were more likely to follow the advice being given, they also reported being less likely to purchase such an information device. This may be the result of less frequent drivers feeling that the savings gained from such a device would not outweigh the costs because of their limited driving. Conversely, more experienced and more frequent drivers perceive a net gain and respond as more likely to purchase a device although they do not follow the advice as often. The ANOVA also revealed that drivers will follow advice to take the freeway more readily than advice to take the side road and that they are quicker to respond to freeway advice, indicating that a "route bias" exists.

Analysis of the route choice decision times of (frivers found that there was a very rapid drop in the decision times over the first 8 of 32 trials and that the times remained relatively constant over the remaining 24 trials. This finding and the fact that average acceptance rates of advice approximated the accuracy of the system indicate that drivers could sense and adapt quickly to the level of accuracy being provided by the system. Average decision times were the greatest for information provided at 75 percent accuracy. This indicates that subjects were more readily able to identify the level of ac2 curacy for low levels as well as high levels but took a greater amount of time to discern the moderate level of accuracy.

The efforts to develop a model of route choice behavior that incorporates the learning processes of drivers had mixed results. A model was developed that included drivers' updated perceptions of route delay and information accuracy, but the model was not significantly different from a model that excluded these perceived attributes. The model includes the advised route as a variable. Because subjects followed the advice so readily, the model may simply be predicting that subjects will select the advised route, therefore predicting about 79 percent correct, which is equivalent to the average acceptance rate of advice. More analysis is required using different updating schemes before conclusive results can be made about the effects of experiences on sequential trials: Future research efforts will include attempts to formulate more

realistic information updating schemes and to extend the research and modeling effort to a more realistic traffic network environment.

ACKNOWLEDGMENTS

The authors wish to thank the California Department of Transportation and the Partners for Advanced Transit and Highways for funding this research.

REFERENCES

1. M. A. Abdel-Aty, K. M. Vaughn, R. Kitamura, and P. P. Jovanis, Impact of ATIS on Drivers' Travel Decisions: A Literature Review. Research Report. UCD-ITS-RR-92-7. Institute of Transportation Studies, University of California, Davis, 1992.

2. P. Bonsall and M. Joipt. Driver Compliance with Route Guidance Advice: The Evidence and its Implications. Society of Automotive Engineers, Warrendale, Pa., Oct. 1991, pp. 47-59.

3. P. Bonsall. The Influence of Route Guidance Advice on Route Choice in Urban Networks. Special Issue. Proc., Japanese Society of Civil Engineers, Feb. 1991.

4. P. Bonsall and T. Parry: A Computer Simulation Game to Determine Drivers' Reactions to Route Guidance Advice. Proc., 18th, PTRC Summer Annual Meeting, London, 1990.

5. J. Neter, W. Wasserman, and M. H. Kutner. Applied Linear Statistical Models. IRWIN, Homewood, Ill., 1990.

6. K. M. Vaughn, M. A. Abdel-Aty, R. Kitamura, and P. P. Jovanis. Experimental Analysis of Sequential Route Choice Behavior. Under ATIS. Research Report UCD-ITS-RR-92-16. Institute of

Transportation Studies, University of California, Davis, 1992. 7. H. Yang, R. Kitamura, P. P. Jovanis, K. M. Vaughn, and M. A. Abdel Aty. Exploration of Route Choice Behavior with Advanced Traveler Information Using Neural Network Concepts. In Transportation, Kluwer Academic Publishers, 1993.

8. M. Ben-Akiva and S. R. Lerman, Discrete Choice Analysis Theory and Application to Travel Demand. The MIT Press, Cambridge,

Publication of this paper sponsored by Committee on Transportation Supply Analysis.

TRANSPORTATION RESEARCH RECORD 1408

Automobile during los

Network Performance Under System Optimal and User Equilibrium Dynamic Assignments: Implications for Advanced **Traveler Information Systems**

HANI S. MAHMASSANI AND SRINIVAS PERTA

A comparative assessment was undertaken of network cost and performance under time-dependent system optimal (SO) and user equilibrium (UE) assignment patterns, with particular reference to the effectiveness of advanced traveler information systems (ATIS). Both SO and UE solutions were found using a new simulation based algorithm for the time dependent assignment problem. Experiments were conducted using a test hetwork with signal-controlled junctions under progressively increasing network loading intensities. A diagnosis of system performance; for various intensities of loading was effected using network-level traffic descriptors for both SO and UE assignments. The results affirm the validity of a meaningful demarcation between SO and UE assignments in urban traffic networks and provide useful insights for macroscopic network-level relations, among traffic descriptors. These results suggest that ATIS information supply strategies based on SO-route guidance could considerably outperform descriptive noncooperative information strategies, especially at moderate to high congestion levels in the network, The results also illustrate the time-dependent nature of the gains achieved by an SO assignment vis-à-vis à UE assignment in a congested traffic network.

Approaches incorporating advances in communication technologies, information processing systems, electronics, and automation, broadly labeled as intelligent vehicle highway sys, tems (IVHS), continue to generate considerable interest for their potential to alleviate urban and suburban congestion of traffic systems. Advanced traveler information systems (ATIS) provide travelers with real-time information on existing traffic conditions or instructions on route selection from their current location to their destinations, or both. Successful implementation of ATIS, especially at high market penetration levels, involves the dynamic assignment of vehicles to "optimal" paths to reduce overall system user costs. Recently Mahmassani and Peeta (1) proposed a heuristic algorithm to solve the system optimal (SO) dynamic traffic assignment problem for the ATIS context, in which a central controller with known or predicted time-dependent origin-destination (O-D) trip desires over the horizon of interest solves for paths to prescribe to users to attain some systemwide objectives. A comprehensive review and discussion of dynamic assignment and traffic simulation models for ATIS-advanced traveler management system (ATMS) applications are given by Mahmassani et al. (2).

Department of Civil Engineering, The University of Texas at Austin, Austin, Tex. 78712-1076.

In this paper, the performance of a traffic network entploying this solution methodology is analyzed for both SO and user equilibrium (UE) time-dependent assignments. As in the static case, SO and UE dynamic assignments involve similar algorithmic steps, differing primarily in the specification of path travel costs that form, the basis of the cotresponding assignments. SO dynamic assignment is accomplished using time-dependent marginal travel times [see:Ghali and Smith (3)], whereas a UE assignment is attained using the time-dependent average travel times. The system performance under the above assignment schemes was analyzed for various intensities of network loading covering the spectrum of network states from uncongested networks to very highly congested networks. In addition, the numerical experiments illustrate the extent of the differences between SO and UE time-dependent assignments in terms of total system cost at varying levels of network congestion. This question is of fundamental importance to ATIS operations, with regard to the relative benefits of normative versus descriptive infor-'mation supply strategies.

'An SO assignment does not generally represent an equi-·librium flow pattern because some users may be able to obtain "individual advantages simply by changing routes, though they may impose a greater marginal cost on other users in the system in the process. Its significance to the ATIS context lies in providing a benchmark against which other assignments or flow patterns can be gauged, thereby yielding an upper bound on the benefits attainable with real-time traffic information. A Wardrop UE holds when users cannot improve their individual costs by unilateral route switching. There is no empirical evidence that UE conditions hold in real networks, although the UE solution is considered a reasonable and useful construct for the evaluation of long-term capacity improvements. Under real-time descriptive ATIS information on network conditions, a time-dependent UE pattern could be viewed as the result of the long-term evolution of the system, as users somehow learn and adjust under the supplied information. However, it is not at all clear that such convergence would be attained under inherently dynamic conditions (exacerbated by supplying information to users). Thus, it is not known what the UE solution may represent from the 'standpoint of ATIS operation and evaluation. Actual user behavior and system performance under real-time descriptive information may be better or worse than the corresponding

time-dependent UE solution in terms of the overall system cost. Nevertheless, a time-dependent UE pattern may be considered as a useful proxy for a favorable scenario of longterm network performance under real-time descriptive information.

It is known from static network equilibrium theory that, e SO and UE lead to identical solutions only for situations in taking the arithmetic average of the 5-min averages. Similarly, which the shortest paths taken by users simultaneously are the best paths from a system viewpoint. Such situations are observed when networks are relatively uncongested so that link operating speeds are unaffected by the flows on the links... (limited vehicle interactions). At the other extreme, under highly congested conditions, system performance is not likely to be markedly different under the two assignment schemes because the opportunities for SO to sufficiently ameliorate the traffic situation probably would be limited.

For network conditions between the two extremes, the extent of the differences between SO and UE solutions, particularly in terms of overall system cost, is not known. This is very important for ATIS because if the two solutions are anot perceptibly different, coordinated cooperative SO route guidance imposed by a central controller may not be necessary, and descriptive information that is less complicated and simpler to disseminate to noncooperating drivers may be sufficient. The similarity of the two solutions would have important implications for the focus that ATIS information supbly strategies should take, with more attention directed to ways of guiding the system toward UE convergence and away from wide fluctuations. However, if SO indeed holds promise for meaningful gains over UE, then normative route guidance or strategies to induce the system near its SO should be pursued. It is also desirable to ascertain network and traffic conditions under which differences between SO and UE are meaningful.

In this paper, overall user cost and network performance under time-dependent SO and UE assignment patterns are examined in a series of numerical experiments performed on a test network under various loading levels. The system performance is gauged using average network-level traffic flow descriptors, in addition to the standard parameters such as «average travel time. The time-dependent nature of the prob-·lem further complicates the already intricate problem of charracterizing traffic flow performance at the network level that was previously addressed only under steady-state conditions, as discussed hereafter.

NETWORK TRAFFIC FLOW THEORY

Mahmassani et al. (4,5) generalized the definitions of speed, flow, and concentration to the network level and examined their interrelation in their model of network traffic performance. These concepts are extended to the dynamic case in the current analysis to characterize the vastly varying network traffic conditions (especially for medium to high network loading levels) during the peak period. Average network speed V (kilometers per hour) is obtained as the ratio of total vehicle kilometers to total vehicle hours in the network over the duration of interest. The average network concentration K (vehicles per lane kilometers), for the duration of interest, is the time average of the number of vehicles per unit lane length in the system. However, the concentration varies dramatically with time in dynamic traffic networks. Hence, the timedependent network concentration is examined by taking 5-min averages of the number of vehicles per unit lane length in the system. An overall measure of network concentration K over the duration of the period of interest is obtained by time-dependent network flow, interpreted as the average number of vehicles per unit time that pass through a random point along the network, is examined by taking 5-min averages; an overall measure of network flow Q over the peak period is obtained by taking the simple average of $(\sum l_i q_i)$ (Σl_i) , where q_i and l_i , respectively, denote the 5-min average flow and the length of Link i, and the summations are taken over all network links.

Two fundamental relationships between these three network traffic flow variables are investigated in this study. The first relates average network speed (V) and average network concentration (K). For arterials or single roadways, a qualitative trend of decreasing speed with increasing concentration is well established. The same general frend was observed to hold at the network level in the simulation experiments of Mahmassani et al. (5), although the complexity of network interactions preclude the analytic derivation of such a relation directly from the link-level relations. The second relationship analyzed is the basic identity Q = KV. Formally established for single roadways, it was shown to also hold at the network level in the previously mentioned steady-state experiments (5). These experiments were performed keeping the network concentration level constant for the duration of interest by treating the network as a closed system. The NETSIM package was used for the study, and vehicular behavior was governed by the comprehensive microscopic rules embedded in NETSIM. The present study replicates the network traffic conditions of a rush hour traffic situation. It uses the DY-NASMART (DYnamic Network Assignment Simulation Model for Advanced Road Telematics) simulation-assignment model developed at The University of Texas at Austin for ATIS-ATMS applications. The Q = KV identity is expected to hold only approximately for time-varying network traffic flow.

SOLUTION METHODOLOGY

Problem Statement

Consider a traffic network represented by a directed graph G(N, A) where N is the set of nodes and A the set of directed arcs. A node can represent a trip origin, a destination, or a junction of physical links. Consider a network with multiple origins and destinations. The time experienced by a vehicle to traverse a given link depends on the interactions taking place among vehicles in the traffic stream along this arc. The analysis period of interest, taken here as the peak period, is discretized into small equal intervals $t = 1, \dots, T$. Given a set of time-dependent O-D vehicle trip desires for the entire duration of the peak period, expressed as the number of vehicle trips r_{iii} leaving Node i for Node j in time slice t; $\forall i$; j $\in N$ and $t = 1, \ldots, T$, determine a time-dependent assignment of vehicles to network paths and corresponding arcs. In other words, find the number of vehicles rin that follow path $k = 1, \ldots, K_{ij}$ between i, and j at time t, $\forall i, j \in N$ and t $= 1, \ldots, T$, as well as the associated numbers of vehicles on each arc $l \in A$ over time. As explained in the previous section, two such assignments are computed: (a) one that satisfies UE conditions that no user can improve actual (experienced) trip time by unilaterally changing routes and (b) one that minimizes total travel time (for all users) in the system over the peak period. The interpretation of these two solutions from the standpoint of ATIS effectiveness was discussed in the previous section.

Simulation Assignment Solution Procedure

This section describes briefly the algorithm used to solve for SO and UE assignments. A detailed description of the solution procedure by Mahmassani and Peeta (I) consists of a heuristic iterative procedure in which a special-purpose traffic simulation model is used to represent the traffic interactions in the network and thereby evaluate the performance of the system under a given assignment. As indicated earlier, the algorithmic steps for UE assignment are virtually identical to those for the SO solution except for the specification of the appropriate arc costs and the resulting path processing component of the methodology. The algorithm is first summarized for the SO case, followed by a brief description of the modification for the UE problem.

The use of a traffic simulation model to evaluate the SO objective function and model system performance circumvents the principal difficulties that have precluded solutions' to realistic formulation of the problem by obviating the need for link performance functions and link exit functions and implicitly ensuring that the first-in, first-out property holds on traffic facilities and that no unintended holding back of traffic takes place at nodes [see Mahmassani et al. (2) for a discussion of issues arising in dynamic traffic assignment]. The algorithm uses the DYNASMART simulation assignment model. DYNASMART has the capability to simulate the movement of individual vehicles through the network, with path selection decisions possible at every node or decision point along the way to the destination, as supplied by the user decision rules reflecting driver behavior in response to realtime information. In this work, vehicular paths are preassigned exogenously to DYNASMART, as determined by the steps of the SO or UE solution algorithms. Thus DYNA-SMART is used primarily as a simulator to replicate the dvnamics of traffic phenomena in response to a given assignment of vehicles to paths. A detailed description of the various capabilities of DYNASMART is provided by Mahmassani et al. (6).

The simulation results provide the basis for a directionfinding mechanism in the search process embodied in the solution algorithm for this nonlinear problem. The experienced vehicular trip times from the current simulation are used to obtain a descent direction for the next iteration. The time-dependent shortest travel time, paths and least marginal travel time paths are obtained using the time-dependent algorithms described by Ziliaskopoulos and Mahmassani (7). An elegant aspect of the solution methodology is that it avoids" complete path enumeration between O-D pairs.

Figure 1 depicts the solution algorithm for the SO dynamic traffic assignment problem. A brief summary of the approach is as follows:" "

- 1. Set the iteration counter I = 0. Obtain the time-dependent historical paths (paths obtained from the data base) for each *assignment time step over the entire duration for which the assignment is sought.
- 2. Assign the O-D desires (which are known a priori for the entire peak period) for the entire duration to the given paths and simulate the traffic pattern that results from the assignment using DYNASMART.
- 3. Compute the marginal travel times on links using timedependent experienced or estimated link travel times and the number of vehicles on links obtained as post-simulation data (from Step 2).
- 4. Using a special-purpose, time-dependent, least-cost path algorithm, compute the least marginal time paths for each O-D pair for each assignment time step on the basis of the marginal travel times obtained in Step 3.
- -5. Perform an all-or-nothing assignment of O-D desires to the least marginal time paths computed in the previous step. The result is a set of auxiliary path vehicle numbers for each O-D pair for each assignment time step $t = 1, \dots, T$.
- χ 6. Update paths and the number of users assigned to those paths. Paths are updated by checking whether the path identified in Step 4 already exists (i.e., has carried vehicles in at least one prior iteration) for that O-D pair and including it if it does not. The update of the number of vehicles (assignment of yehicles to the various paths currently defined between the O-D pair after the path update) is performed using the method of successive averages (MSA), which takes a convex combination of the current path and corresponding auxiliary path numbers of vehicles for each O-D pair and each time step. A detailed description of MSA is provided by Sheffi and Powell (8). Note that other convex combination schemes could be used equally.
- *7. Check for convergence using an ε-convergence criterion. 8. If the convergence criterion is satisfied, stop the program. Otherwise, update the iteration counter I = I + 1 and go to Step 2 with the updated data on paths and the number of vehicles assigned to each of those paths.

The complexity of the interactions captured by the simulator when evaluating the objective function generally precludes the kind of well-behaved properties required to guarantee convergence of the algorithm in all cases. However, such convergence was achieved in all the experiments reported in this paper and in many other test networks solved to date. Also, path marginals are not necessarily global because they are based on link level marginal travel times. Efforts were made to attain a global optimum where local solutions were suspected.

Modification To Obtain User Equilibrium Solution

As previously discussed, the solution to the time-dependent UE problem is obtained by assigning vehicles to the shortest average travel time paths instead of the least marginal paths in the direction-finding step (Step 5). In other words, use the

FIGURE 1 Solution algorithm for the SO dynamic assignment problem

(time-dependent) average travel times on links instead of the marginal travel times in the shortest-path calculations. In the above solution procedure, this simplifies Step 3 and modifies Step 4 as indicated.

EXPERIMENTAL DESIGN AND SETUP

Network Configuration and Traffic Characteristics

The test network used in this study, having 50 nodes and 163 links, consists of a freeway with a street network on both sides, as shown in Figure 2. Nodes within the freeway section are neither origin nor destination nodes. A total of 38 origin nodes and 38 destination nodes are obtained by excluding freeway nodes (Nodes 1 through 37 and 44). Freeway nodes are connected to the street network through entrance and exit ramps. Unless otherwise indicated in Figure 2, all arcs shown *are two directional. All links are 0.83 km (0.5 mi) long and have two lanes in each direction, except for the entrance and exit ramps, which are directed arcs with a single lane. The

freeway links have a mean free speed of 91.67 km/hr (55 mph); and the other links have a mean free speed of 50 km/hr (30 mph). In terms of traffic signal characteristics, 25 intersections have pretimed signal controls, 8 have actuated signal controls, and the remaining 17 nodes have no signal control.

Experimental Setup

The comparative assessment of system performance for SO and UE assignments is conducted under various network loading levels, which generate various levels of network congestion. The network loading factor is defined as the ratio of the total number of vehicles generated in the network during the assignment period to a given reference number (19,403 vehicles over a 35-min period in the experiments). Table 1 shows the various loading factors considered in this study and the corresponding number of vehicles generated on the test network during the duration of interest (35 min in all cases). In addition, it shows the corresponding number of "tagged" vehicles (vehicles generated for the 30-min duration after the

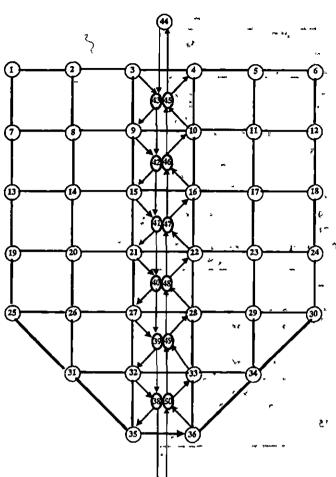


FIGURE 2 Network structure.

Mahmassani and Peeta

5-min start-up time) for which relevant performance statistics are accumulated. The loading factors range from 0.6 (very low congestion with 11,616 vehicles) to 2.4 (extremely high congestion with 46.674 vehicles). Under each loading level. the UE and SO solutions are obtained, and the resulting timedependent link flow patterns are obtained from DYNA-SMART. The shape of the loading curve for the various network loading levels emulates real-world-network loading for . the peak period, with an initially increasing generation rate until a peak is reached, followed by a decreasing vehicle generation rate.

In the present study, a start-up time of 5 min is provided in DYNASMART for the network to be reasonably occupied. followed by a 30-min peak period generation of traffic (for which performance statistics are accumulated). Another aspect of the experimental setup that critically influences the system performance is the spatial distribution of the O-D demand pattern. The vehicles generated are about evenly distributed spatially in terms of both their origins and destinations, except for Nodes 37 and 44, which generate or attract only about 25 percent of the number of vehicles originating or destined to a typical O-D node (i.e., Nodes 1 through 36).

The results from the various experiments are viewed from two principal perspectives. First, they form the basis for com--parison of system performance, particularly user costs under UE and SO assignment schemes, thereby addressing the questions relevant to ATIS information strategies described in the introductory section of the paper. Second, they are used to investigate network-lèvel traffic flow characteristics and relations using network-wide traffic descriptors. This investigation is conducted primarily for the SO flow pattern. An additional element of the study is the time-dependent analysis of the travel time gains of SO over UE, also of significance to ATIS operation.

The results provide several key insights from both of the above perspectives. They manifest a clear qualitative and quantitative distinction in the solution provided by the SOassignment scheme as opposed to the time-dependent UE assignment procedure to route vehicles in a general traffic network. The results also reveal important and robust macroscopic relationships among network-level traffic variables that parallel those for single roadways.

System Performance for SO and UE Assignments

Table 1 reports summary statistics on the system performance for the SO and UE assignments for the various loading factors. As expected, at low levels of network loading, when the network is relatively uncongested, the average travel times of vehicles in the network are relatively close across the various loading levels. As the load is increased, the effects of congestion become more prominent and the average travel times in the network increase at an increasing rate with the loading factor. At very high loading levels, the marginal effect of additional demand on system performance is very high. The results also indicate that there is only limited variation in the average distance traveled by vehicles under the various network loading levels, implying that greater congestion and not longer travel routes is the primary cause of the higher system trip times (the objective function seeks to minimize total sys-'tem travel time only). Nevertheless, the average travel distance does increase with the loading level, reflecting an increasing percentage (although small in magnitude) of drivers assigned to longer travel routes. The average travel distances under UE for various network loading levels are smaller than the corresponding distances for SO, indicating a smaller percentage of long travel routes under UE. This may be explained by some users being assigned to longer routes to reduce congestion elsewhere to reduce systemwide travel times.

Figure 3 shows comparatively the average trip times under various network loads for UE and SO assignments. As discussed above, both curves illustrate the increasing marginal effects of additional demand on system trip times. Of more relevance to the central question addressed in this paper, Figure 3 highlights the difference in the quality of the solutions provided by the two assignment rules for time-dependent network flows. This is further illustrated in Figure 4, which depicts-the percentage improvement in average travel time of SO over UE (as'a fraction of the UE travel time) for the various average network concentrations corresponding to the TABLE 1 Summary Statistics for: SQ and UE Assignments

	<u> </u>		* 4 4		_^		4 1
Loading	No. of	No. of	Average	Total	Average	Total	Average
Factor	Generated Vehicles	Tagged Vehicles	Trip Time	Trip Time	Trip Distance	,Ţrip "Distance	Speed
		,	(minutes)	(hours)	×(km)		(kmph) 1
System O	ptimal	. ×	*	3			1
, <u>Q</u> ,60	, 11616	10585	₂ .3.85	679. <u>\$</u>	3.03	32096.2	47.23
√0.80 _°	15509	14098	3.90	916,0	3.02	4241146	46:30
, 1.00 ₅	19403	17621	4.03,	1183.1	~ 3.03" •	53391.6	45.22
1.20,	、23305 ↔	21145₽	4:40	1549 5	*3.07	64728.8	41.77/
1.40	27196	24697	· 4.86	1999.1	3.08	76207.9	38.12
1:60	31090	×28205	6.04 3	2837.1	3.20 %	້ 90222.1	31.80
1.80	34978	31726 **	7.65	4042.9	3.28	103997.5	25,72
2.00	38871	22220	10.46	6149.5	3.32	117013.3	19.03
2.10	40818	37014	13.08	8071.9	3.35	123996.7	15.37
2.20	42769*	38784"	16.57.	-10710.9°	3:32	"~12881J".7 <u>.</u>	12,03
2.40	46674	42322 "	24.95	17601.7	3.55	149978.3	8.52
Úser Equi	ilibrium.		F , 4		×	* 15	,
0.60	11616	10585	3.86 -1	681.5	3.00	§ 31839.6	46,72 ; *
0.80	15509	14098	3.92	920.8	. 2.97	41898:3	45:50 ****
1.00	19403	·17621 +	4.15	1219.5	* ₂ 2.98 ³	52656.2	43.18
1.20	a23305	21145	4.60	1622.5	[*] 3.02	63731.2	39.28
1.40 4	27196	24697	5.43	2236.5	3.00	74289:6	33.22
1.60	31090	-28205	£ 6.79	3192.2	3.08	**************************************	, 27.30
1.80	34978	31726	9.00	4762.9	3.13	99513.3	20.883
2.00	38871	35258	^12.91 _*	7587.7	3.27	115249.2	15.18
2.10	40818	37014	14.94	92,15.7	3.22	119132.5	12.93
2.20	42769	38784	18.55	11993.6	3.32		10.72
NOTE: 1 k	m = 0.6 mile	******	, y , y	<u> </u>			7
Ĺ		تعلا	-3"				•

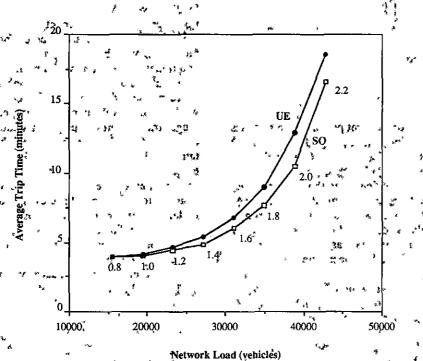


FIGURE 3 Comparison of average trip times (minutes) of SO and UE assignments for various levels of network loading. The number by each *plotted point is the corresponding loading factor.

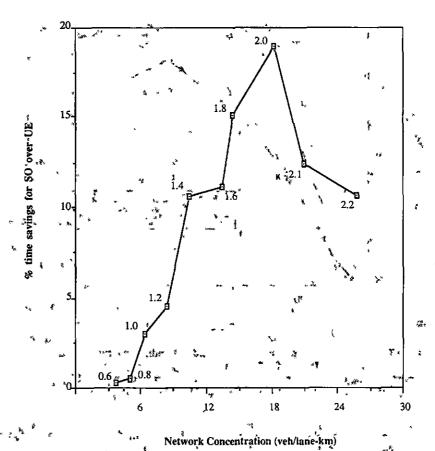


FIGURE 4 Percentage total trip time savings of SO over UE tobtained fraction of total UE trip time for various loading factors versus average network conceptration. The number by each plotted point is the corresponding loading factor.

various levels of network loading. At low loading levels, SO and UE provide essentially identical solutions. For loading factors 0.6 and 0.8, SO shows improvements of 0.3 and 0.5 percent, respectively, over UE. At such low concentration levels, average link speeds remain relatively unchanged because of limited interactions among vehicles, and the marginal tually yielding negligible differences in the quality of the sotravel time on the link is essentially identical to the average travel time, leading to almost identical solutions under the two assignment schemes. When network confestion increases. slightly to loading factors of 1.0 and 1.2, the corresponding SO trip time improvements are 3.0 and 4.5 percent, respectively, over the UE solution. As the network becomes moderately congested, system benefits under the SO assignment become more pronounced, with 10.6 and 11.2 percent improvements over UE for loading factors of 1.4 and 1.6, respectively. For heavily loaded networks, very substantial gains are obtained, with 15.1 and 19.0 percent improvements in system travel times using SO for loading factors 1.8 and 2.0,

As the levels of network loading are increased further, the system reaches very high levels of congestion that near gridlock, and overall network throughput drops, making it increasingly difficult to discharge all vehicles from the system in a reasonable amount of time. Under these conditions, the ability to improve overall conditions by rerouting certain vehicles to paths with lower marginal costs diminishes as all

links become highly congested. Thus, the advantage of an SO assignment relative to UE begins decreasing, as reflected by - reduced improvements of 12.4 and 10.7 percent for loading factors of 2.1 and 2.2, respectively. The gains begin dropping rapidly beyond this point, with higher loading levels evenlution provided by the two schemes.

Figure 5 depicts the comulative demand generation as a function of time under the 2.0 loading factor along with the cumulative discharge curves under the SO and UE assignments. The various points on the plot are obtained by accumulating the statistics available for each 5-min interval. The area on the plot between the two discharge curves represents the time savings of SO over UE—in this case about 1,438 hr. The figure illustrates the time-dependent nature of the benefits generated by SO over UE. When the network is in the early stages of loading (for about the first 20 min), it is not sufficiently congested to produce meaningful differences between SO and UE assignments. Most of the savings of SO are accrued between 30 and 70 min into the peak period as *the network is close to peak congestion levels. Beyond 70 , min, there appear to be virtually no significant gains of SO over UE as the network is again relatively uncongested. Thus the benefits of route guidance based on SO assignment over UE routing are not accumulated uniformly over time—rather they are gained when the network is relatively well congested.

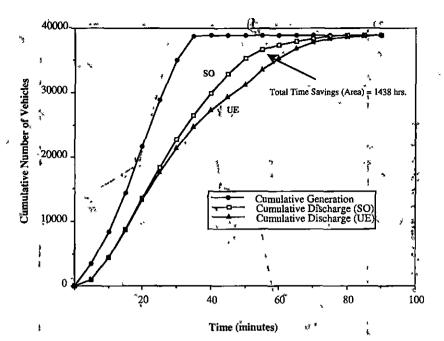


FIGURE 5 Cumulative generation curve and SO and UE cumulative discharge curves for a loading factor of 2.0. The points on the curve-represent 5-min updates of the cumulative number of vehicles. The area between the SO and UE discharge curves represents the time savings for SO over UE.

Figure 6 shows the time savings per vehicle for SO over. UE as a function of the vehicle's time of departure under. various loading factors. To capture the time dependency of. the benefits in a systematic manner, travel time savings are the savings at an increasing rate as the loading level inaccumulated on the basis of the start times of the vehicles." In the figure, 0-5 on the y-axis (start time) refers to all vehicles that start between 0 and 5 min. Vehicles that start during

the first 5 min do not face congested conditions, and hence SO does not yield savings over UE for these vehicles. Vehicles * that start during the intervals 10-15 and 15-20 min accrue creases. Over their trip, these vehicles encounter significant congestion that increases with the loading factor. For vehicles starting between 20 and 35 min, the benefits increase with

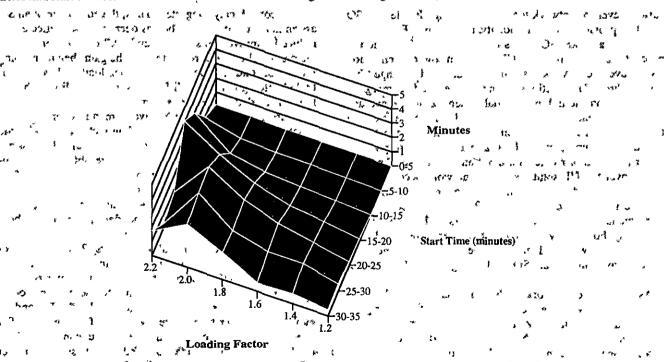


FIGURE 6 'Trip time savings (SO relative to UE) per vehicle (in minutes) as a function of loading factor and start times (in minutes), of vehicles.

network loading at an increasing rate until the 2.0 loading factor level and then dip down. This trend illustrates the previously discussed tendency of diminished savings for SO under extremely high congestion conditions.

Network Flow Relations

The second aspect investigated through the experimental re-/ sults relates to the macroscopic network-level traffic theoretic relationships among network-wide traffic descriptors for dvnamic traffic networks under consideration. The pertinent traffic variables and their averages over time and space were defined in the first section of the paper. As noted, although mathematical relationships among traffic flow variables are reasonably well established for arterials and intersections, the intricacies of interactions at the network level-preclude analytic derivability of network-wide traffic relationships from, the link-level traffic models. However, the simulation results extend the previous findings of Mahmassani et al. (4.5) that the basic trends captured by the single roadway relationships seem to also hold at the network level for the dynamic case.

The network level speed-concentration relationship for the SO assignment is shown in Figure 7. Each point on the plot corresponds to a simulation run for the whole assignment period under a particular loading level. The figure clearly illustrates tlecreasing average network speed with increasing network concentration, paralleling the K-V relationship for an individual roadway. Note that the plot has a point of in flection that corresponds approximately to the 1.8 loading factor. This qualitative trend has been observed previously in the simulation experiments of Mahmassani et al. (4) on a regular test network using the NETSIM package.

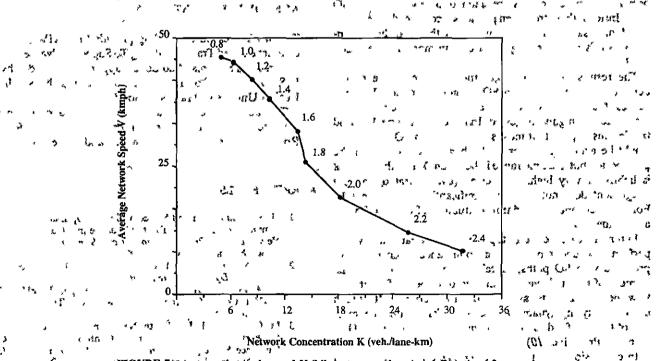
An essential element to be noted in the network-level analysis is the time-dependent nature of the phenomena of inter-

est.' Averaging quantities such as network flow and concentration over the duration of the peak period are likely to mask the time dependency of network performance. For example, overall network concentration is obtained by averaging low levels of concentration at both ends of the peak period and high levels in between, as shown in Figure 8, which shows the time-dependent variation of concentration [normalized by dividing by a jam concentration of 96 vehicles per lane-km (160 vehicles per lane-mile)] over the duration of interest. More detailed investigation of the interrelationships among network-level traffic descriptors over time will be reported elsewhere.

CONCLUSIONS &

The experiments performed using the simulation-based algorithm to solve both the SO and UE versions of the timedependent traffic assignment problem have provided insights of critical importance to the design of ATIS information supply strategies and results of fundamental significance in the context of network assignment and network traffic flow theories: Of course, experimental results from a single-test network preclude definitive generalizations; nevertheless, they offer an illustration of the insights that can be obtained on the basic constitution of the problems being addressed while suggesting directions for future research. The first main conclusion is that the results suggest meaningful differences in overall system cost and performance between time-dependent SO and UE assignments. The second main conclusion is that traffic networks under time-dependent traffic assignment pat terns' continue to operate within the envelope of relatively simple network traffic flow relationships that exhibit strong similarities to the traffic models established for individual road sectións 3.5 11

3 .11 1 1



*FIGURE 7 Average network speed V (kilometers per hour) as a function of average network concentration K (vehicles per lane kilometer) for the SO case.

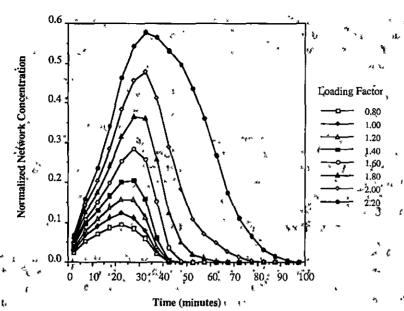


FIGURE 8 Normalized network concentration (network concentration as a fraction of network jam concentration) as a function of time for various loading factors:

If we take the UE assignment results as somehow-indicative of the situation that might be attained over time in a system in which drivers have access to real-time on-board descriptive information through ATIS, the results of our experiments suggest that there is considerable potential for SO, coordinated route guidance, especially in heavily congested (although not oversaturated) networks. These results appear to contradict unsupported claims that descriptive information would likely perform as well as normative SO route guidance because UE system costs were claimed to be very close to SO costs. Instead, they strengthen previous recommendations [e.g., by Mahmassani and Jayakrishnan (9)] that coordinated information is necessary beyond a certain market penetration level.

The results further highlight the dynamic nature of the benefits accumulated by an SO assignment over UE. They suggest that SO is most effective when the traffic network is moderately to highly congested. In the context of peak period traffic, this implies that most savings through SO assignment would be achieved neither at the beginning nor end of the peak period but in a time range in between. When the network is lightly or very highly congested (oversaturated), an SO assignment does not perform significantly better than UE. For relatively uncongested traffic situations, SO and UE yield almost identical solutions.

Future research on this topic will investigate the system performance under partial user compliance when users are provided with SO paths, thereby introducing an additional element of user behavior. With regard to the traffic network flow theoretic aspects, avenues for future efforts in this area include analyzing dynamic traffic networks from the perspective of the two-fluid theory of town traffic developed by Herman and Prigogine (10).

In conclusion, it is possible to characterize traffic flow in urban traffic systems using relatively simple macroscopic re-

lationships that parallel traffic flow relationships at the individual roadway level. Simulation is an abstract representation of real-world traffic; thus the research is mostly exploratory rather than definitive in pature. Results to date strongly suggest that the performance of dynamic traffic networks is critically sensitive to network topology, and network loading pattern.

ACKNOWLEDGMENTS

This work is based on funding provided by FHWA under a contract entitled Traffic Modeling To Support Advanced Driver Information Systems. Additional support was provided by the U.S. Department of Transportation through the Southwest Region University Transportation Center.

The authors are grateful to Ta-Yin Hu and Athanasios Ziliaskopoulos for their essential contributions and to Richard Rothery for his beneficial comments and suggestions.

REFERENCES

- H. S. Mahmassani and S. Peeta. System Optimal Dynamic Assignment for Electronic Route Guidance in a Congested Traffic Network. Proc., 2nd International Capri Seminar on Urban Traffic Networks, Capri, Italy, July 1992.
- H. S. Mahmassani, S. Peeta, G. Chang, and T. Junchaya. A Review of Dynamic Assignment and Traffic Simulation Models for ATIS/ATMS Applications. Technical Report DTFH61-90-R-00074-1. Center for Transportation Research. The University of Texas at Austin, April 1992.
- 3. M. O. Ghali and M. J. Smith. New Dynamic Model to Evaluate the Performance of Urban Traffic Control Systems and Route Guidance Strategies. In *Transportation Research Record 1306*, TRB, National Research Council, Washington, D.C. pp. 33-39, 1991

- 4. H. S. Mahmassani, J. C. Williams, and R. Herman. Investigation of Network-Level Traffic Flow Relationships: Some Simulation Results. In *Transportation Research Record 971*, TRB, National Research Council, Washington, D.C., pp. 121-130, 1984.
- H. S. Malimassani, J. C. Williams, and R. Herman. Performance of Urban Transportation Networks. In Transportation and Traffic Theory. (N. H. Gartner and N. H. M. Wilson, eds.), Elsevier Science Publishing Co., Inc., 1987.
- 6. H. S. Mahmassani, T. Hu, and R. Jayakrishnan. Dynamic Traffic Assignment and Simulation for Advanced Network Informatics (DYNASMART). Proc., 2nd International Capri Seminar on Urban Traffic Networks, Capri, Italy, July 1992.
- A. Ziliaskopoulos and H. S. Mahmassani. Design and Implementation of a Shortest Path Algorithm with Time-Dependent Arc Costs. Proc., 5th Advanced Technology Conference, Washington, D.C., Dec. 1992.
- 8. Y. Sheffi and W. Powell. An Algorithm for the Equilibrium Assignment Problem with Random Link Times. *Networks*, Vol. 12, pp. 191-207, 1982.
- 9. H. S. Mahmassani and R. Jayakrishnan. System Performance and User Response under Real-Time Information in a Congested Traffic Corridor. Transportation Research 25A, pp. 293-308, 1991.

 10. R. Herman and I. Prigogine. A Two-Fluid Approach to Town Traffic Science, Vol. 204, pp. 148-151, 1979.

The results and views expressed in this paper are solely the responsibility of the authors and do not necessarily reflect the sponsors official opinion.

Publication of this paper sponsored by Committee on Transportation Supply Analysis.